

# The structural design of Kunshan Xintiandi super high-rise office building

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**Abstract:** The Kunshan urban investment project includes 1 office tower, 4 high-rise residential buildings and 1 high-rise commercial building. Each monomer shares a large basement chassis, a total of three basement floors. This design is the second phase (5# super high-rise and corresponding basement). The height of the structure is more than 140 meters, which is a concrete-filled steel tubular frame + core tube system. The selection and structure of the structural system are analyzed, and the mechanical performance of the structure is verified through calculation and analysis.

## 1 Project Overview

The construction site of the project is located in Kunshan City, Jiangsu Province. The proposed site is located on Qianjin West Road to the north, Louchuang Road to the

south and Sichang Road to the east. Zu Chong Road to the west. The project includes one office tower, four residential towers and one commercial tower. Each unit shares the basement, which has three floors in total. This design is 5# super high-rise office and corresponding basement.



Figure 1. Architectural renderings.

The design life of the project is 50 years, the safety level of the building structure is grade 2, the fire level of the building structure is grade 1<sup>[1]</sup>, and the design level of

the foundation foundation is grade a. According to the "Code for Seismic Design of Buildings" (GB50011-2010) (2016 edition) <sup>[2]</sup>, the aseismic group 1 in the area where

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the project is located, the site category is category IV, the aseismic grade is 7 degrees 0.1g, and the office area is general fortification category (category c). The load refers to Load code for the design of building structures<sup>[3]</sup>.

## 2 Structure selection and classification

The project is located in the 7 degree of seismic fortification intensity area, belongs to the standard fortification class building, should be 7 degrees of seismic calculation and seismic measures.

In this project, the height of the tower structure is 142 meters (30 stories), the main structure of the concrete filled steel tube frame-shear wall core structure system. As the main lateral force resistance system, the core tube not only provides resistance to wind load and horizontal earthquake action, but also bears the additional torsion effect caused by the incoincidence of the center of mass and the center of stiffness. The outer frame mainly bears the vertical load, and also bears the role of the second seismic defense line. Due to the high height of the first floor and the second floor, combined with the requirements of seismic structure, the thickness of the shear wall in the bottom strengthening area is 600mm and 500mm, and the thickness of the upper shear wall is reduced to 400mm and 300mm. The seismic bearing capacity and structural ductility of the shear wall can be improved by increasing the stirrup ratio of the constrained edge members. The outer frame column is reinforced concrete column, and the frame column connected with the cantilever is reinforced concrete column. The exterior floor of the above-ground core tube is mainly reinforced truss floor bearing plate. Typical floor thickness: basement roof 180mm;The thickness of the composite floor of the profiled steel plate is 120mm;The floor thickness of the core tube is 150mm, and the floor thickness of the other parts is 120mm.

### 2.1 The setting of structural joints and post-cast belt

#### 2.1.1 Shrink post-cast strip

The plane size of the basement of this project is 118mx470m, which is a super-long structure.

In order not to affect the use function of the building, this project does not set expansion joints, according to the "Concrete Structure Design Code" [2] (GB50010-2010) using every 30m ~ 40m interval to set up shrinkage post-pouring belt construction measures, reduce the early

concrete shrinkage of the adverse impact on the structure. The width of the post-pouring strip is 0.8m. Shrinkage post pouring belt on both sides of the members of the concrete pouring for two months, most of the concrete shrinkage after the completion of supplementary pouring. The non-shrinkage concrete with strength grade higher than that of the two side members is used for the supplementary casting of the post cast belt.

The following construction measures and construction measures are adopted in the design to reduce the adverse effects of temperature action and concrete shrinkage in the later period on the structure:

(1) The reinforcement should be as close as possible, and the proportion of the reinforcement should be increased appropriately.

(2) Strengthen the insulation measures of the skirt roof layer and the top floor of the main building, and set the outer insulation layer on the outer wall.

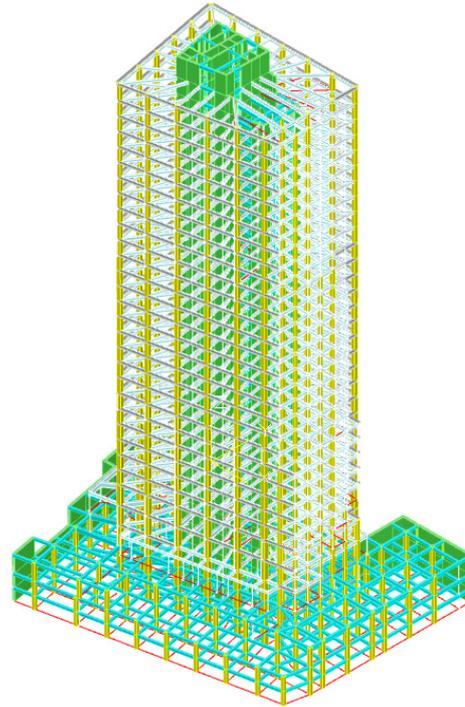
(3) Use of low shrinkage, low hydration heat of cement, appropriate to reduce the amount of cement. Add proper amount of fly ash to concrete.

### 2.2 Settlement joint

There are many differences in the number of floors and loads between the main building, the basement and the podium of the project, which causes uneven settlement of the foundation. In order not to affect the use function of the building, the project does not set up the settlement joint, the design of the settlement after pouring belt construction measures to reduce the adverse impact of foundation differential settlement. The settlement post-pouring belt is located at the junction of the main building and the skirt house, on the side of the skirt house, with a width of 0.8m. It should be poured after the structure is capped and the foundation settlement is basically stable. The non-shrinkage concrete with strength grade higher than that of the two side members is used for the supplementary casting of the post cast belt.

## 3 Calculation and analysis

According to the technical code for concrete structures of high-rise buildings (JGJ 3-2010) <sup>[1]</sup>, The 3d analysis and design software YJK (version 3.0.3) were used for the structural analysis and design<sup>[5]</sup>. The influence of floor cavity is considered in the analysis. The superstructure is embedded in the basement roof of the first floor, The earthquake action and the wind load act in two main directions and the torsional effect under the two-way earthquake action is taken into account.



YJK calculation model

**Figure 2** finite element analytical model

This project uses YJK as the main tool for structural design and calculation. Table 1 shows the main data of the northwest high-rise building:

**Table 1.** Two types of software modal cycles and vibration types

Vibration model	Period (s)	Vibration type	period ratio [0.90]
	YJK		
1	3.379	Y	0.83
2	1.92	X	
3	1.73	torsion	

**Table 2.** Interlayer displacement Angle comparison of software

		YJK result	
		Interlayer displacement Angle	displacement ratio
small earthquakes	X	1/1165	1.14
	Y	1/1529	1.26
	Specification limits	1/800	1.4

## 4 Conclusion

After analysis, the overall design index is good, the first and second period is translational period, the third period is torsional period, the stiffness of the two directions of the structure is not much different, and the ratio of the first torsional period to the first translational period of the structure is less than 0.85, which meets the requirements of the code. The shear weight ratio of the two directions at

the bottom of the structure meets the requirements of not less than 1.2% in the "Anti-Gauge".

Both directions were less than the standard limit of 1/800. The calculation models of the two software are basically consistent and the dynamic characteristics of the structure are basically consistent. The natural vibration period, displacement Angle between layers, displacement ratio and other indexes of the structure are similar, and all of them are in a reasonable range, which meet the requirements of the current code.

## References

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