

Numerical studies of soil base deformations from reconstructed multi-storey building to nearby buildings

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Abstract. During the building reconstruction with floors addition, there is a need to evaluate the building frame and foundation soil bearing capacity, especially if there are deviations from the design parameters. As a result of the field and numerical studies, we determined the basic change patterns in the stress-strain state of the 12-storey building load-bearing structures with a monolithic reinforced concrete frame due to uneven pile foundation deformation. We also found the influence degree of the existing deviations from the design parameters to the structures bearing capacity. The research results can be applied in reconstruction conditions with a significant increase in the load on the existing load-bearing elements of the building and on the pile foundation.

Keywords. Groundbase, pile-plate foundation, bored pile, bearing capacity, monolithic frame.

1 Introduction

To date, the pile-raft foundations are one of the most effective developments in the foundation engineering field [1-19]. They successfully apply in the design of multi-storey facilities on sites with unfavorable geological conditions, so far as they have an increased bearing capacity [2, 20-24]. However, in recent times, it is quite often in the construction of such foundations, there are cases of deviation from design solutions, which can lead, despite their high distribution capacity, to the uneven settlements development and, as a result, damage to the above-ground structures [1, 3, 9-14]. In such conditions, it is important to carry out full-scale and numerical researches of the building load-bearing frame stress-deformed state in order to assess their residual resource and the necessity for reconstruction (strengthening). A large number of works are devoted to the reconstruction of buildings and structures [4, 15-19]. But these sources do not consider the influence of the foundation with piles of various lengths on the above-ground structure behavior. In this regard, it is relevant to consider the building model in the form of a single system «building – foundation – soil», taking into account the immediate features of the pile foundation.

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2 Materials and methods

The researched twelve-story administrative building was erected at the beginning of the 21st century. In terms it has a square shape with beveled corners. The building is made with a rigid load-bearing frame system from monolithic reinforced concrete, with reinforced concrete beam slabs and a stiffening core, where passenger and freight elevators are placed. The building foundation is designed from the pile-raft made from reinforced concrete bored piles with a diameter 800 mm and reinforced concrete raft grillage with a thickness 1200 mm. The pile field plan is shown in Fig. 1.

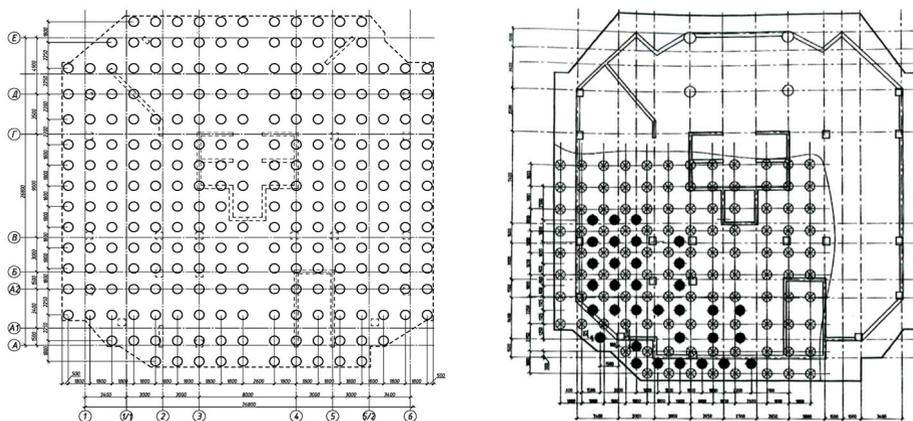


Fig. 1. The pile foundation plan according to the project solution and strengthening scheme with soil-cement pile.

The pile-raft foundation during construction was performed with significant deviations from the project – under a part of the building (in axes 1-4 / A-g) piles have a length of 5.4 m instead of a design of 10 m. Implementation of the pile field with the indicated deviations led to the following changes: the soil base uneven deformation and settlement; the raft grillage uneven and additional deforming; the tilt both of the raft grillage and the building as a whole. Due to the uneven deformation and tilt of the raft grillage in the building load-bearing structures, there were additional (unaccounted in design) forces and stresses, which led to the formation and development of cracks in the frame elements and to the deformed (non-design load) framework performance scheme during operation. Due to the deformations and building tilt in the short piles direction, the appearance of the cracks in the frame crossbars because of non-calculated tensile stresses, in the future foundations we introduced strengthening in the short piles zone with a soil-cement pile with diameter 600 mm and 10 m long (Fig. 1).

Strengthening the pile foundation by the introduction of soil-cement piles made it possible to restore the bearing capacity of the foundation soil to design values. However, this approach has not allowed to eliminate additional settlement, besides this, the elasticity modulus of soil-cement piles is significantly less than the elasticity modulus of reinforced concrete piles, which leads to the emergence and development of additional foundation soil settlement, additional movements and efforts in the frame load-bearing structures and in the raft grillage. When forecasting load-bearing capacity and the evaluation of the forecasted building residual life for reconstruction and subsequent operation it is necessary to take into account the factors mentioned above.

2.1 Construction site engineering and geological conditions

The engineering and geological conditions of the building site are characterized by inconsistency on the soils layers strike in the area and the depth, the inhomogeneity of their

composition and state. The research area refers to the Bugulminsk-Bellebevs elevated plateau with deep erosion dismemberment. The site, where the building is located, is situated within the left-bank terrace of the valley St. Zai River. According to the engineering and geological survey results, used in project development of the building pile foundation, alluvial-deluvial quaternary deposits and Upper-Permian eluvial deposits are included into the geological structure of the site. From the surface they are covered with industrial deposits of modern Quaternary Period (Fig. 2). During surveys, underground waters are not detected.

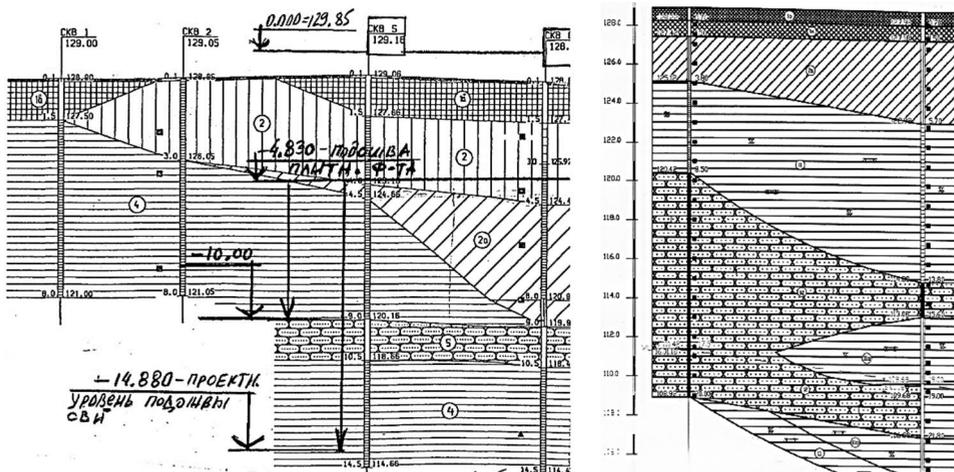


Fig. 2. Engineering and geological cross-section 2000 and 2018.

The geological and lithological structure of the site includes:

- technogenic bulk soils that are distributed over the entire platform, the thickness of bulk soils up to 1.6 m;
- clay loam from a solid to a low-plasticity consistency with a thickness from 2 to 7.5 m, which have settlement properties at the top of the soil profile;
- clay argillite-like, solid consistency, with a thickness of up to 7.3 m;
- sandstone, fine grain, low strength.

The research conducted in the study of the building stress-strain state in 2018 indicates the increase in humidity and deterioration in the clay loam state. In the surveys, there were revealed two irregular groundwater levels – in the Quaternary and Upper-Permian deposits. Groundwater is unconfined, the aquifer recharge is achieved by atmospheric precipitation infiltration, the upward groundwater discharge from the underlying layers. Besides, a comparative analysis of the results conducted in different years shows that the soil layers depth location, their classification and characteristics differ from each other.

2.2 Results of the field survey of the building

During a field survey in 2018 it was found that the maximum cracks width in reinforced concrete beams and inter-floor slabs did not increase, but there was an increase in the crack width, which previously had a less cracks width, and also there was a formation of new cracks between existing ones. The nature of the crack's development (Fig. 3) indicates simultaneous action on beams and slabs from bending moments and longitudinal tensile loads in the lower floors, and the longitudinal compressive loads in the upper floors, causing additional disclosure of cracks. The cracks development in the bearing slabs indicates the continuing deformations of the soil mass between piles, soil-cement piles and the settlement of pile foundation.

It should be noted that the grade of concrete in the structures of the surveyed building is

sufficiently high. The compressive strength conforms to grade C25-B45 (design concrete grade B25). The measurement results are presented in Table 1. The strength checkout was carried out by the shock pulse monitoring in accordance with GOST 22690-88 by an electronic shock-pulsed meter of the strength IPS-MG4.03.

Table 1. Results of the concrete compressive strength measurement of the reinforced concrete structures.

No.	Construction name	Construction coordinates	Type of concrete	\bar{R} , MPa	Concrete grade
1	Columns	basement, 13 floor	heavy	25.1-31.3	B20-B25
2	Beams	basement, 1-13 floors	heavy	28.9-47.1	B25-B40
3	Slabs	basement, 1-13 floors	heavy	34.7-52.2	B30-B45
4	Walls	basement	heavy	24.5-29.2	B20-B25

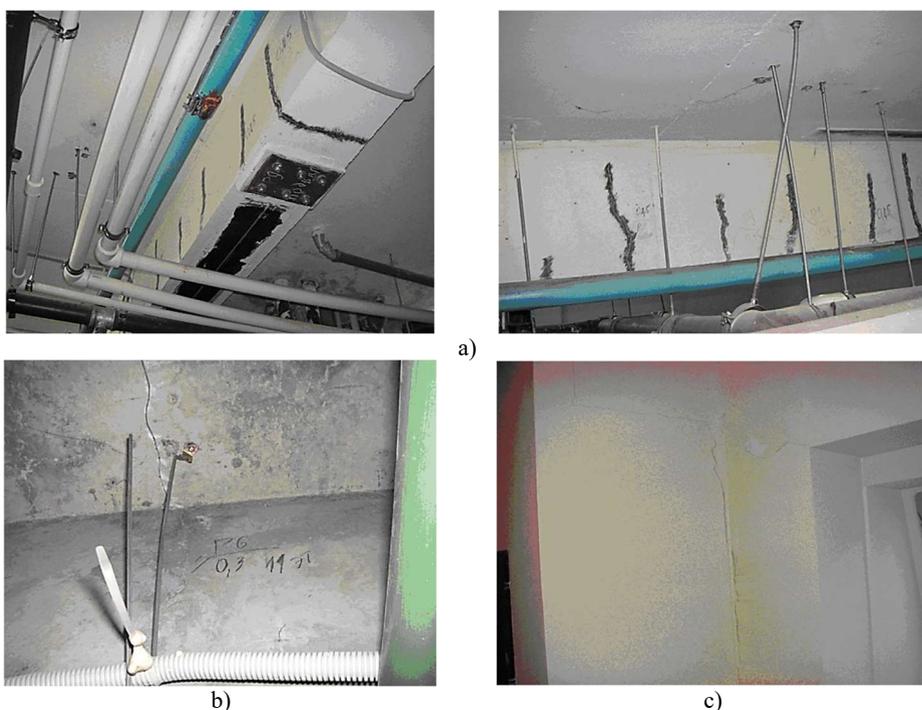


Fig. 3. Cracks: a) in beams; b) in slabs; c) in walls.

2.3 Numerical studies of the building frame stress-strain state

For evaluation of the building system bearing capacity, the numerical studies using the Complex of Calculation Programmes such as Lira-CAD were additionally conducted. As noted in [5-8, 11, 14], the building soil bases undergo a complex stress state, which affects the super-structure stress-strain state. Therefore, to obtain the calculation results, which are the most representative of a real behavior of the building, it is necessary to calculate the unified system «pile foundation – raft grillage – a multi-storey frame». There were created two spatial models in the calculations:

- Model 1 – model of the building frame in design state. Based on this model, there was determined a stress-deformed state of the frame, followed by checking its bearing capacity in

the design geometric and mechanical characteristics of structural elements, foundations and soils without regard to the base strengthening of the pile foundation and the presence of defects and damage in the bearing and enclosing structures in the design loads on the building;

- Model 2 – model of the building frame, shows its actual state at the pursuance of the research. Based on this model, there was calculated a structural building system, including the pile base strengthening, the presence defects and damage in the frame bearing elements.

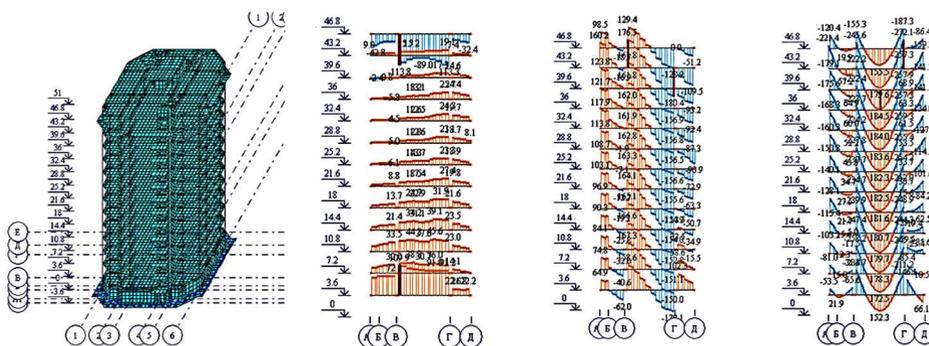


Fig. 4. Spatial analytical model of the frame and the nature of the distribution of the loads N , q , M in the beams section 300×800 height along the building under the static loads (model 2).

The designed models represent a system in which columns, reinforced concrete wall envelopes – shell elements, raft grillage – elements of a thick shell. The analytical frame model is presented in Fig. 4. The presence of defects and cracks in structures was performed by introducing a reduced concrete deformation modulus, determined in accordance with the SP 63.13330.2012 «Concrete and reinforced concrete structures». The work of piles was simulated using the semigrad element system (bracings), where the rigidity was taken depending on the soil deformation characteristics and the parameters of the piles. The calculation of the yielding bracing stiffness was made on the basis of the formula [25-27]:

$$K = P/s_G, \tag{1}$$

where P is the load on the pile, s_G – the pile settlement from the load P , taken into account the influence of the neighboring piles according to the SP 52-102-2003 «Design and constitution of pile foundations»: $s_G = R_s s_1$ (R_s – settlement increasing coefficient; s_1 – settlement of a single pile). The yielding bracing stiffness calculation was made for three types of piles:

- *BNS 10-80* – design bored pile 10 m long and 800 mm diameter;
- *BNS 5.4-80* – bored pile of non-design length (5.4 m) and 800 mm diameter;
- *CGS 10-60* – strengthen ground-cement pile 10 m long and 600 mm diameter.

4 Results

The calculation results are shown in Table 2. The non-design length pile (5.4 m) stiffness was 6343.48 kN, which is 35% lower than the stiffness of the design pile (10 m). Considering the presence of collapsible loam clay under the raft grillage and the possibility of forming a gap on the junction between the raft and the soil when the latter is soaked, the entire load from the building was transferred to piles in the models.

The calculation was made on the action of permanent and temporary loads in accordance with the set of rules SP 20.13330.2016, SP 14.13330.2014. For accounting the strengthening stage, there was used the «Montaj» Module, which is installed into the architecture of the Lira-CAD software. Additionally, there were written codes in the C + language to make an evaluation of the frame bearing capacity with wind and seismic impact. The results of calculation were presented in tables 3-7 and in Fig. 4-7.

Table 2. Summary table of pile calculation.

Type of pile	BNS 10-80	BNS 5.4-80	CGS 10-60
Pile load bearing capacity F_d , kN	1412.762	922.842	897.303
Pile Permissible load N_d , kN	1009.116	659.173	640.935
Pile settlement S_G , m	0.103	0.104	0.0621
Stiffness of the yielding bracing that simulates the work of piles K, kN/m	9726.606	6343.48	10305.6

Table 3. Summary table of ultimate loads on piles (with consideration to strengthen base).

Model	In design length piles (10 m)			In non-design length piles (5.4 m)		
	permissible load, N_d , kN	N_{max} , kN	N_{min} , kN	permissible load, N_d , kN	N_{max} , kN	N_{min} , kN
Model 1	1009.116	986.7	533.1	659.173	-	-
Model 2		809.8	655.3		514.1	383.6

Table 4. Piles strength utilisation ratio.

Type of pile	BNS 10-80 (design)		BNS 5.4-80 (non-design)	
Permissible load on the pile N_d , kN	1009.116		659.173	
Alternative design	Design fulfillment	Actual state (with strengthening)	Design fulfillment	Actual state (with strengthening)
Pile maximum load N_{max} , kN	986.7	809.8	-	514.1
Piles strength utilization ratio $K = N_{max}/N_d$	0.977	0.802	-	0.78

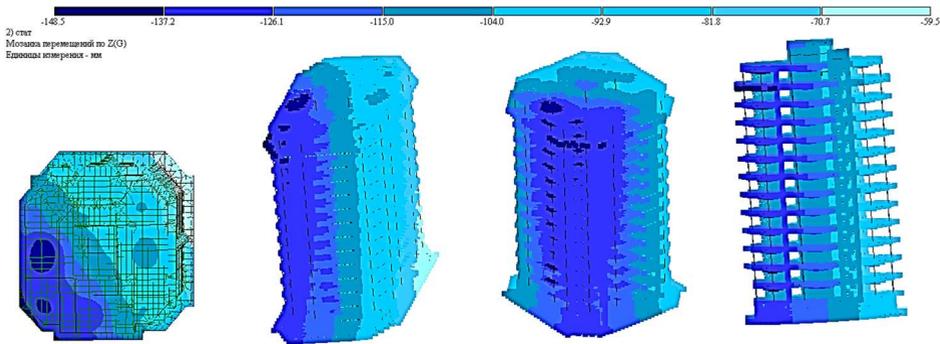


Fig. 5. Frame strain state under the static loads (model 2).

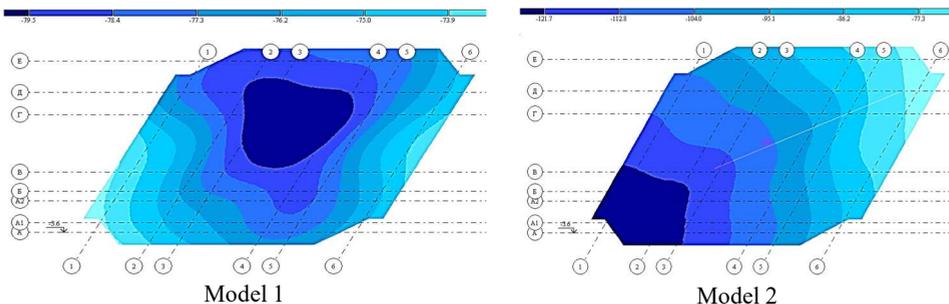


Fig. 6. Raft grillage strain state in the design (model 1) and the actual (model 2) states.

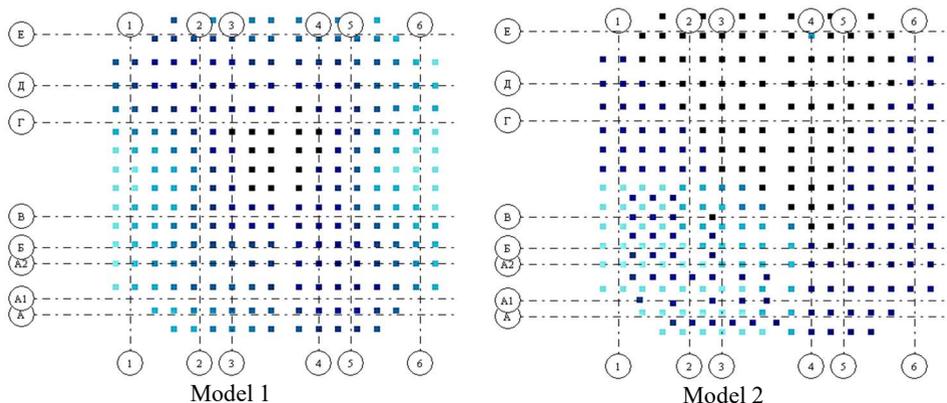


Fig. 7. Loads distribution mosaic on piles in model 1 and model 2.

Table 5. Calculated settlements.

Model	Maximum settlement, mm	Minimum settlement, mm	Mean settlement, mm	Tilt of the foundation
Design state (model 1)	80.7	71.6	76.15	0.00024
Actual state (model 2)	130.7	59.5	95.1	0.00019
Limit values (SP 22.13330.2016)	-	-	150.0	0.003

Table 6. Column strength utilisation ratio.

Floor number	Design state (model 1)		Actual state (model 2)	
	K_{max}	K_{min}	K_{max}	K_{min}
1 st floor	0.886	0.379	0.972	0.449
7 th floor	0.589	0.383	0.652	0.372
13 th Floor	0.885	0.120	0.901	0.144

Table 7. The most loaded floor beams strength utilisation ratio.

Floor number	Beams cross section 300×500 mm				Beams cross section 300×800 mm			
	Design fulfillment (model 1)		Actual State (model 2)		Design fulfillment (model 1)		Actual State (model 2)	
	support	span	support	span	support	span	support	span
1	0.862	0.986	0.859	0.982	0.862	0.986	0.859	0.982
7	0.74	0.921	0.733	0.987	0.74	0.921	0.733	0.987
13	0.588	0.931	0.667	0.987	0.588	0.931	0.667	0.987

4 Conclusion

The authors presented numerical studies of the system «pile foundation – raft grillage – a multi-storey frame». Taking into account all elements joint deformation, made it possible to find the stress-strain state changing patterns of all system elements during long-term operation. The main cause of non-stationary deformation of reinforced concrete frame elements, raft grillage and pile base, is the presence of a zone with non-design length piles, and reduction of the rigidity of the base to 35% in this zone. As a consequence, there is a redistribution of the load between the pile base and reinforced concrete elements of the frame and, as a result, there are non-design additional loads in the slabs and columns, and also the building tilt increases. In monolithic reinforced concrete slabs, not only bending moments are applied, but also longitudinal tensile and compressive stresses arise, which is confirmed by the formation of new and development of existing normal and inclined cracks in these elements.

Despite the above-mentioned processes occurring in the elements of the system «pile foundation – raft grillage – a superstructure», the bearing capacity of the combined raft-pile foundation, vertical and horizontal bearing frame elements is sufficient to perceive the existing operational loads. The combined raft-pile foundation settlements and tilts are in the design standards permissible limits.

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